WHT PLATE C CONCRETE



PLATES FOR TENSILE LOADS

TWO VERSIONS

WHT PLATE 440, ideal for framed structures (platform frame); WHT PLATE 540, ideal for CLT panel structures (Cross Laminated Timber).

PLANAR JOINTS

Ideal for realizing distributed connections under tensile stress between the CLT (Cross Laminated Timber) panels and framed structures (platform frame) to and the concrete understructure.

QUALITY

The high tensile strength allows to optimize the number of plates installed, ensuring remarkable time saving.

Values calculated and certified according to CE marking.



CHARACTERISTICS

FOCUS	tensile joints on concrete					
HEIGHT	EIGHT 440 540 mm					
THICKNESS	3,0 mm					
FASTENERS	LBA, LBS, SKR, VIN-FIX, HYB-FIX					



MATERIAL

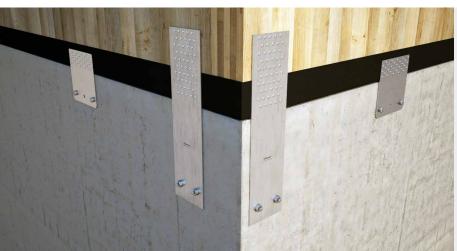
Bright zinc plated carbon steel, two dimensional perforated plate.

FIELDS OF USE

Timber-to-concrete shear joints for panels and timber struts

- CLT, LVL
- solid timber and glulam
- framed structures (platform frame)
- timber based panels





TIMBER-TO-CONCRETE

Beside its natural function, it is ideal for solving situations where the transfer of tensile loads from timber to concrete is required.

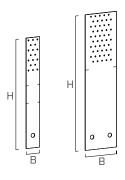
MULTIPURPOSE

Pre-calculated partial nailing can be used if there is a varying amount of stress or a levelling layer.

CODES AND DIMENSIONS

WHT PLATE C

CODE	В	Н	holes	n _v Ø5	s		pcs
	[mm]	[mm]	[mm]	pcs	[mm]	A . A . A	
WHTPLATE440	60	440	Ø17	18	3	•	10
WHTPLATE540	140	540	Ø17	50	3	•	10



MATERIAL AND DURABILITY

WHT PLATE C: carbon steel DX51D+Z275.

To be used in service classes 1 and 2 (EN 1995-1-1).

EXTERNAL LOADS

FIELD OF USE

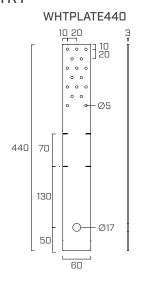
- Timber to concrete joints
- OSB to concrete joints
- Timber-to-steel joints

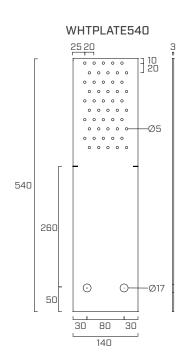
ADDITIONAL PRODUCTS - FASTENING

type	description		d	support
			[mm]	
LBA	Anker nail	<u>[</u>	4	
LBS	screw for plates	(D41111111111++	5	
AB1	mechanical anchor		16	
VIN-FIX(*)	chemical anchor		M16	
HYB-FIX	chemical anchor		M16	
KOS	bolt		M16	

 $^{^{(*)}}$ For more information, see the data sheet available at www.rothoblaas.com

GEOMETRY



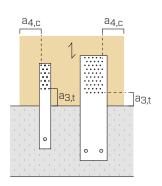




INSTALLATION

TIMBER minimum distances			nails LBA Ø4	screws LBS Ø5
C/GL	a _{4,c}	[mm]	≥ 20	≥ 25
C/GL	a _{3,t}	[mm]	≥ 60	≥ 75
CIT	a _{4,c}	[mm]	≥ 12	≥ 12,5
CLT	a _{3,t}	[mm]	≥ 40	≥ 30

- C/GL: minimum distances for solid timber or glulam consistent with EN 1995–1-1 according to ETA considering a timber density $\rho_k \le 420 \text{ kg/m}^3$
- CLT: minimum distances for Cross Laminated Timber according to ÖNORM EN 1995-1-1 (Annex K) for nails and ETA 11/0030 for screws



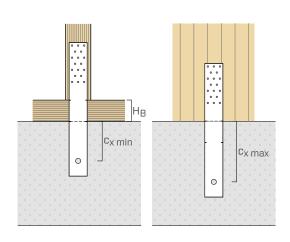
WHTPLATE440 INSTALLATION

The WHT PLATE 440 can be used for different construction systems (CLT/frame) and ground connection systems (with/without platform beam, with/without levelling layer). Depending on the presence and dimension of $H_{\rm B}$ of the intermediate layer, in accordance with the minimum distances of the timber and concrete fasteners, the WHT PLATE 440 must be positioned in way that the anchor is at a distance from the concrete edge:

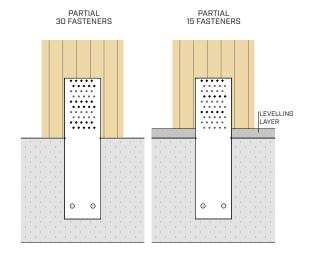
130 mm $\leq c_x \leq 200$ mm.

WHTPLATE540 INSTALLATION

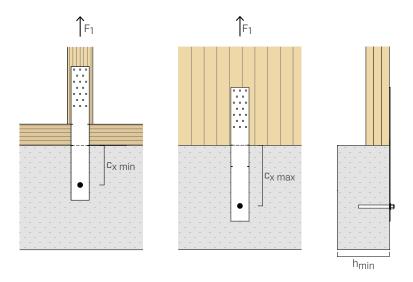
In the presence of design requirements such as varying stress values or the presence of a **levelling layer** between the wall and the support surface, it is possible to use pre-calculated and optimised **partial nailing** in order to influence the effective n_{ef} number of fastenings on timber. Alternative nailings are possible in accordance with the minimum distances for the connectors.



C _X	H _B
[mm]	[mm]
c _{x min} = 130	70
$c_{x max} = 200$	0



■ STRUCTURAL VALUES | TENSILE JOINT | TIMBER-TO-CONCRETE WHTPLATE440



$MINIMUM \ CONCRETE \ THICKNESS \ h_{min} \ge 200 \ mm$

	R _{1,K} TIMBER				R _{1,K} S	TEEL	R _{1,d} CONCRETE					
configuration	holes	fastening Ø5		R _{1,k timber}	R _{1,k}	steel	R _{1,d uncrack}	ed	R _{1,d cracked}		R _{1,d seismic}	:
	type	ØxL	n _v				VIN-FIX 5.8 Ø x L		VIN-FIX 5.8 Ø x L		HYB-FIX 8.8 Ø x L	
		[mm]	[pcs]	[kN]	[kN]	Ysteel	[mm]	[kN]	[mm]	[kN]	[mm]	[kN]
• c _{2 min} = 130 mm	LBA nails	Ø4,0 x 60	18	35,0	34,8		M16 × 10F	22,6	M16 x 195	16,0	M16 x 195	16.0
 total fastening 2 anchors M16	screws LBS	Ø5,0 x 60	18	31,8	34,0	Үм2	M16 x 195	22,0	MT0 X 132	10,0	M10 X 195	16,0
• c _{2 max} = 200 mm	LBA nails	Ø4,0 x 60	18	35,0	74.0		M16 × 10F	72.7	M16 v 10F	22.0	M16 × 10F	22.0
 total fastening 2 anchors M16	screws LBS	Ø5,0 x 60	15 ⁽¹⁾	27,5	34,8	Үм2	M16 x 195	32,3	M16 x 195	22,9	M16 x 195	22,9

MINIMUM CONCRETE THICKNESS $h_{min} \ge 150 \text{ mm}$

		R _{1,K} TIMB	ER		R _{1,K} S	TEEL	R _{1,d} CONCRETE					
configuration	holes	fastening Ø5		R _{1,k timber}	R _{1,k}	steel	R _{1,d uncracke}	ed	R _{1,d cracked}		R _{1,d seismic}	С
	type	ØxL	n _v				VIN-FIX 5.8 Ø x L		VIN-FIX 5.8 Ø x L		HYB-FIX 8.8 Ø x L	
		[mm]	[pcs]	[kN]	[kN]	Ysteel	[mm]	[kN]	[mm]	[kN]	[mm]	[kN]
• c _{2 min} = 130 mm • total fastening	LBA nails	Ø4,0 x 60	18	35,0	34,8	V	M16 x 130	18,2	M16 x 130	12,9	M16 x 130	12,9
• 2 anchors M16	screws LBS	Ø5,0 x 60	18	31,8	34,0	Үм2	M10 X 130	10,2	MIO X 130	12,9	MIO X 130	12,9
• c _{2 max} = 200 mm	LBA nails	Ø4,0 x 60	18	35,0	7/10	N.	M16 v 170	26.0	M16 v 170	10 /	M16 v 170	10 /
 total fastening 2 anchors M16	screws LBS	Ø5,0 x 60	15 ⁽¹⁾	27,5	34,8	Үм2	M16 x 130	26,0	M16 x 130	18,4	M16 x 130	18,4

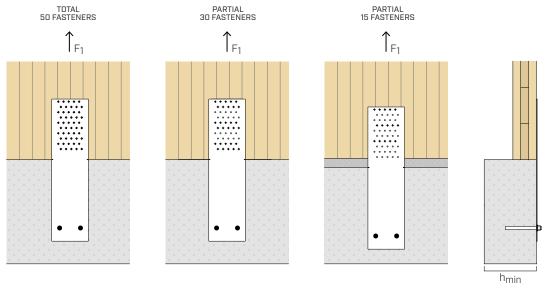
NOTES:

 $^{^{(1)}}$ For the configuration in the table it is recommended not to install the screws of the lower row at a distance of $a_{3,t}$ (stressed end) = 15d = 75 mm.



■ STRUCTURAL VALUES | TENSILE JOINT | TIMBER-TO-CONCRETE

WHTPLATE540



MINIMUM CONCRETE THICKNESS $h_{min} \ge 200 \text{ mm}$

		R _{1,K} TIMB	ER		R _{1,K} S	,K STEEL R _{1,d} CONCRETE ⁽³⁾						
configuration	holes	fastening Ø5		R _{1,k timber}	R _{1,k}	steel	R _{1,d uncracke}	d	R _{1,d cracked}		R _{1,d seismic}	:
	type	ØxL	n _v				VIN-FIX 5.8 Ø x L		VIN-FIX 5.8 Ø x L		HYB-FIX 8.8 Ø x L	
		[mm]	[pcs]	[kN]	[kN]	Ysteel	[mm]	[kN]	[mm]	[kN]	[mm]	[kN]
• total fastening	LBA nails	Ø4,0 x 60	50	83.5								
• 2 anchors M16	screws LBS	Ø5,0 x 60	50	81.6								
• partial fastening ⁽²⁾ 30 fasteners	LBA nails	Ø4,0 x 60	30	70,8	70.6		M16 x 195	44,1	M16 x 195	31,3	M16 x 195	26,6
• 2 anchors M16	screws LBS	Ø5,0 x 60	30	69.9	70.0	Үм2	MIO X 193	44,1	MIO X 193	31,3	MIO X 193	20,0
• partial fastening ⁽²⁾ 15 fasteners	LBA nails	Ø4,0 x 60	15	35,4								
• 2 anchors M16	screws LBS	Ø5,0 x 60	15	35,0								

MINIMUM CONCRETE THICKNESS $h_{min} \ge 150 \text{ mm}$

		R _{1,K} TIME	BER		R _{1,K} S	STEEL		R _{1,d} CONCRETE ⁽³⁾					
configuration	holes type	fastening Ø5 Ø x L	n _v	R _{1,k timber}	R _{1,k}	steel	R _{1,d uncracke} VIN-FIX 5.8 ØxL	d	R _{1,d cracked} VIN-FIX 5.8 ØxL		R _{1,d seismic} HYB-FIX 8.8 ØxL		
		[mm]	[pcs]	[kN]	[kN]	Ysteel	[mm]	[kN]	[mm]	[kN]	[mm]	[kN]	
• total fastening	LBA nails	Ø4,0 x 60	50	83.5									
• 2 anchors M16	screws LBS	Ø5,0 x 60	50	81.6									
• partial fastening ⁽²⁾	LBA nails	Ø4,0 x 60	30	70,8	70.6		M16 v 170	75.0	M16 x 130	25.4	M16 v 170	21.6	
30 fasteners • 2 anchors M16	screws LBS	Ø5,0 x 60	30	69.9	70.6	Үм2	M16 x 130	35,9	MT0 X 120	25,4	M16 x 130	21,6	
• partial fastening ⁽²⁾	LBA nails	Ø4,0 x 60	15	35,4									
15 fasteners • 2 anchors M16	screws LBS	Ø5,0 x 60	15	35,0									

NOTES:



 $^{^{(2)}}$ In the case of configurations with partial nailing, the strength values in the table are valid for the installation of fasteners in timber in accordance with $a_1\!>\!10d~(n_{ef}=n).$

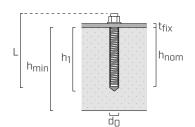
 $^{^{(3)}}$ The concrete strength values are valid if the assembly notches of the WHTPLATE540 plate are positioned at the timber-to-concrete interface ($c_v = 260 \text{ mm}$).

■ CHEMICAL ANCHORS INSTALLATION PARAMETERS^[1]

ancho	or type	t_{fix} $h_{nom} = h_{ef}$		h ₁	d ₀	h _{min}
type	Ø x L [mm]	[mm]	[mm]	[mm]	[mm]	[mm]
VIN FIVE O	M16 x min 130	3	110	115	10	150
VIN-FIX 5.8	M16 x 195	3	164	170	18	200
LIVE FIX O O	M16 x min 130	3	110	115	4.0	150
HYB-FIX 8.8	M16 x 195	3	164	170	18	200

 $INA\ precut\ threaded\ rod\ complete\ with\ nut\ and\ washer:\ see\ INA\ data\ sheet\ at\ www.rothoblaas.com$

MGS threaded rod class 8.8 to be cut to length: see MGS data sheet at www.rothoblaas.com.



t_{fix} h_{nom} h_{ef} h₁ d₀

fastened plate thickness nominal anchoring depth effective anchor depth minimum hole depth

hole diameter in the concrete support concrete minimum thickness

DIMENSIONING OF ALTERNATIVE ANCHORS

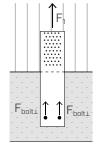
Fastening elements to the concrete through anchors not listed in the table, shall be verified according to the load acting on the anchor and evaluable through the coefficients $kt_{t\perp}$. The lateral shear load acting on the anchor can be obtained as follows:

$$F_{bolt\perp,d} = K_{t\perp} \cdot F_{1,d}$$

 $k_{t\perp}$ coefficient of eccentricity

F₁ tensile stress acting on the WHT PLATE

	$\mathbf{k}_{t\perp}$
WHTPLATE440	1,00
WHTPLATE540	0,50



The anchor check is satisfied if the design tensile strength, obtained considering the boundary effects, is greater than the design external load: $R_{bolt \perp d} \ge F_{bolt \perp d}$.

NOTES FOR SEISMIC DESIGN



Particular attention has to be paid to the "capacity design" applied at different scale levels: the global structure and the connection system. Experimentally the ultimate strength of the LBA nail (and of the LBS screw) is notably larger than the characteristic strength evaluated according to EN 1995.

than the characteristic strength evaluated according to EN 1995. E.g. LBA nail Ø4 x 60 mm: $R_{\nu,k}$ =2,8 - 3,6 kN by experimental tests (variable according to the type of timber and plate thickness).

Experimental data derive from tests carried out within the Seismic-Rev research project and are reported in the scientific report: "Connection systems for timber buildings: experimental campaign to characterize stiffness, strength and ductility" (DICAM - Department of Civil, Environmental and Mechanical Engineering - UniTN).

NOTES:

(1) Valid for the strength values shown in the table.



GENERAL PRINCIPLES:

 Characteristic values according to EN 1995-1-1. The design values of the anchors for concrete are calculated in accordance with the respective European Technical Assessments.

The connection design strength value is obtained from the values on the table as follows:

$$R_{d} = min \begin{cases} \frac{R_{k, timber} \cdot k_{mod}}{\gamma_{M}} \\ \frac{R_{k, steel}}{\gamma_{steel}} \\ R_{d, concrete} \end{cases}$$

The coefficients $k_{\mathsf{mod}}, y_{\mathsf{M}}$ and y_{steel} should be taken according to the current regulations used for the calculation.

- The timber strength values $R_{\rm 1,k~timber}$ are calculated considering the effective number according to Table 8.1 (EN 1995-1-1)

- The calculation process used a timber characteristic density of $\rho_k=350~kg/m^3$ and C25/30 concrete with a thin reinforcing layer and minimum thickness indicated in the relative tables.
- Concrete design strength values are supplied for uncracked ($R_{1.d\ uncracked}$), cracked ($R_{1.d\ cracked}$) concrete and in case of seismic verification ($R_{1.d\ seismic}$) for use of chemical anchor with threaded rod in steel class 5.8.
- Seismic design in performance category C2, without ductility requirements on anchors (option a2 elastic design according to EOTA TR045). For chemical anchors it is assumed that the annular space between the anchor and the plate hole is filled ($\alpha_{gap}=1$).
- The strength values are valid for the calculation hypotheses defined in the table; for boundary conditions different from the ones in the table (e.g. minimum distances from the edge), the anchor-to-concrete group can be verified using MyProject calculation software according to the design requirements.
- Dimensioning and verification of timber and concrete elements must be carried out separately.

